



Mark Young
President

Page Baldwin Jr.
Trustee

Matt Gause
Trustee

Richard Harris
Trustee

Marshall Cook
Trustee

AGENDA

**Meeting of the
Reclamation District 2084
Board of Trustees
Thursday, November 2nd, 2023
8:30 am**

Larsen Wurzel and Associates, Inc.
2450 Venture Oaks Way
Suite 240
Sacramento, CA 95833

Alternative Location:
Office of Page Baldwin, Jr.
3348 Liberty Island Road
Rio Vista, CA 94571

NOTICE TO THE PUBLIC

For Virtual Public Access:
Meeting Link (via Microsoft Teams):
[Click here to join the meeting](#)

Call in:

1-469-294-4078

Meeting number/access code: 157 348 221#

Any member of the public appearing virtually may speak during Public Comment. Reclamation District No. 2084 will use best efforts to swiftly resolve requests for reasonable modifications or accommodations with individuals with disabilities, consistent with the Americans with Disabilities Act, and resolving any doubt whatsoever in favor of accessibility.

- 1. Call to Order**
- 2. Roll Call and Opening Remarks**
- 3. Public Comment (New Business)**

This is an opportunity for members of the public to directly address the Board on subject matter not on the agenda within the jurisdiction of the Board.

- 4. Agenda Approval**
- 5. Consent Items (Action Item)**

- a. Approval of Meeting Minutes

1. October 5th, 2023

Enclosure 1: Agenda Item 5.a.1 – Meeting Minutes

- 6. Board Items (Action item unless otherwise noted)**

There are no board items to address at this meeting.

7. Operations and Maintenance Update (Informational/Action Item)

a. Update from MBK Engineers

Enclosure 2: Agenda Item 7.a – November 2023 Engineer’s Report

b. Ongoing Maintenance Items

8. Financial Management (Informational/Action Item)

a. Invoicing

Enclosure 3: Agenda Item 8.a – October Financial Manager’s Report

9. Little Egbert Project Update (Informational Only)

10. Other Reports (Informational Only)

a. Trustee Report(s)

b. General Manager’s Report

c. Counsel Report (if needed)

11. Adjourn

a. The next regular Board meeting is December 7th, 2023.

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- Any documents related to agenda items that are made available to the Board before the meeting will be available for review by the public by contacting info@rd2084.org.
 - If you need reasonable accommodation due to a disability, please contact info@rd2084.org at least 48 hours in advance of the meeting. This contact information may also be used for any questions you may have.
 - Public comments are generally limited to three (3) minutes but may be more or less at the discretion of the Board.
 - The Board may consider the agenda items listed above in a different order at the meeting, pursuant to the determination of the Board Chair. All items appearing on this agenda, whether or not listed expressly for action, may be deliberated upon and subject to action at the discretion of the Board.

ENCLOSURE 1

AGENDA ITEM 5.a.1



**RECLAMATION
DISTRICT BOARD**

Mark Young
President

Page Baldwin Jr.
Trustee

Matt Gause
Trustee

Richard Harris
Trustee

Marshall Cook
Trustee

MINUTES

**Meeting of the
Reclamation District 2084
Board of Trustees
Thursday, October 5th, 2023
8:30 am**

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1. Call to Order

The meeting was called to order at 8:30am. President Young presided at the meeting.

2. Roll Call and Opening Remarks

Trustees Present: Mark Young, President

Page Baldwin, Jr.

Matt Gause

Marshall Cook

Trustees Absent: Richard Harris

3. Public Comment (New Business)

There was no public comment.

4. Agenda Approval

Trustee Gause moved to approve the agenda.

Trustee Cook seconded and it passed by unanimous vote of Trustees present.

AYES: Baldwin, Cook, Gause, Young

NOES: (none)

ABSTAIN: (none)

RECUSE: (none)

5. Consent Items (Action Item)

a. Approval of Meeting Minutes

1. August 3rd, 2023

Enclosure 1: Agenda Item 5.a.1 – Meeting Minutes

Trustee Cook moved to approve the consent items.

Trustee Gause seconded and it passed by unanimous vote of Trustees present.

AYES: Baldwin, Cook, Gause, Young

NOES: (none)

ABSTAIN: (none)

RECUSE: (none)

6. Board Items (Action item unless otherwise noted)

There are no board items to address at this meeting.

7. Operations and Maintenance Update (Informational/Action Item)

a. Update from MBK Engineers

Enclosure 2: Agenda Item 7.a – October 2023 Engineer's Report

Engineer Moncrief gave an update on District Engineering.

Tina Anderson of MBK has all the information from the District needed for the subventions claim. FEMA coordination is ongoing so the plan is to amend the subventions claim if FEMA funding is realized. There is currently no guaranteed subventions funding after this fiscal year. There is a template language letter available to send to legislators to find long-term funding; letters should be submitted by the end of next month.

b. Ongoing Maintenance Items

Shannon-Wilson investigated active seepage areas: the seepage site north of the Baldwin Property spot and the site North of the main irrigation tide gate. Kevin Tillis recommended deeper exploratory investigation at the sites; the likely outcome will be a seepage berm.

MBK is continuing to work on permits for the erosion site. Sandbags may need to be placed upstream of the site. The Board requested MBK secure a proposal from Asta Construction to stockpile rock in the event that it is needed along the site.

c. Flood Season Preparation

Engineer Moncrief discussed the upcoming flood season. There is warmer and wetter weather in the southern part of the state. This may lead to more runoff with reservoirs closer to capacity than they were last year.

For flood fighting preparation, remember that there is only flood fighting that can be done to a certain point, then there is nothing to be done.

Supplies from the flood fight bin used last year need to be restocked.

d. GSRMA Pump Station Appraisal

Engineer Moncrief has been coordinating with Golden State Risk Management Authority to conduct a pump station site visit for the appraisal. The visit is scheduled for October 23rd.

8. Financial Management (Informational/Action Item)

a. Invoicing

Enclosure 3: Agenda Item 8.a – September Financial Manager’s Report

9. Little Egbert Project Update (Informational Only)

General Manager Nagy provided an update on the Little Egbert Multi-Benefit Project.

LEJPA and CNRA executed an amendment to extend the existing agreement to December 31st, 2025. LEJPA and DWR continue to discuss the scope of work that will be used to further amend the CNRA agreement. DWR and LEJPA are also discussing the scope of work for the agreement between DWR and LEJPA.

AB 345 is sitting on the Governor’s desk to be signed. LEJPA is awaiting action or inaction.

At the most recent LEJPA meeting, the Board requested staff reach out to California Forever. Solano County is meeting with representatives from California Forever today.

10. Other Reports (Informational Only)

a. Trustee Report(s)

None

b. General Manager’s Report

None

c. Counsel Report (if needed)

None

11. Adjourn

a. The next regular Board meeting is November 2nd, 2023.

Trustee Gause moved to adjourn the meeting.

Trustee Cook seconded and it passed by unanimous vote of Trustees present.

AYES: Baldwin, Cook, Gause, Young

NOES: (none)

ABSTAIN: (none)

RECUSE: (none)

The meeting was adjourned at 9:22am.

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ENCLOSURE 2

AGENDA ITEM 7.a

MEMORANDUM

November 2, 2023

TO: Reclamation District No. 2084

FROM: MBK Engineers

SUBJECT: November Engineer's Report

Trustees:

Described below are the items constituting the engineer's report to be discussed at your scheduled November 2023 meeting.

Subventions Program 2022-23 – The DWR fiscal year ended June 30. We are developing your draft claim submittal and will provide a copy for review. Our FEMA coordination is in parallel so we may include some work in the State Claim that may ultimately be pulled if FEMA approves. Final subventions cover letter was sent Late October for District Signature.

Fall-Winter 2023 Activities

- 1) Seepage maintenance: Asta Construction and Shannon Wilson will perform cutoff trenches along seepage areas this November as maintenance action. The same T&M rates will be used as the recent seepage investigation work.
- 2) Erosion monitoring: Rip Rap Stockpiles will be placed near the active erosion site, under the same sole-source contract as seepage repair, to provide local stockpile if site continues to move. We received information from the SUACE regulatory office regarding emergency authorized measures that could be applied to support an emergency repair that we are reviewing.
- 3) Animal control: continue to patrol for rodent activity, sinkholes, voids, and embankment movement.
- 4) Vegetation Control: Any new tree grown (less than 2" in diameter at DBH) should be removed completely.
- 5) Roadway: We will be coordinating with Baldwin to assess roadway conditions to consider placing additional gravel on the levee crown prior to wet weather. There are a few additional sites that may require all-weather road surface to minimize excessive rutting and embankment damage during wet weather levee patrols.

Erosion Slip Repair considerations: the repair of the erosion site on Cache Slough will require in-water work, and regulatory authorization. This will include the following approvals: USACE (NWP,RGP8), RWQCB 401 WQ Cert, CDFW LSAA, and CVFPB Maintenance Notification. The existing protections will need to stay in place for the upcoming flood season.

Seepage Investigation and Repair Considerations: See attached report from Shannon-Wilson regarding the recent seepage investigation.

FEMA/OES Coordination – The District and MBK are wrapping up data entry and project filing with FEMA to support obligation of projects and management review to determine eligibility. There is no certainty on project obligation and funding at this point

Pumpstation assessment October 23 – The Oct 23 inspection did not occur. Golden State is reaching out to Alliant to see what conflicts arose and work to reschedule the pumpstation appraisal. We will be the point people moving forward to support the annual review.

Pre-Season Flood Coordinate Update – Pre-Season flood coordination meeting occurred the same day as the recent earthquake last week. There will be a strong El Nino all winter, forecast, and the probabilities are currently being slanted towards a wetter than normal year. While these are just probabilities, historically strong El Nino's have more often brought wetter weather to the southern California Region. Central Valley region historically has seen 50/50 change of wetter weather. Most significant change from last year is the change in reservoir storage. This time last year the reservoirs upstream had significant capacity compared to this year; see attached graphics.

We are working with district to procure more flood fight supplies to refill DWR containers and provide some additional flood fight supplies for the District. We will be working with landowner/District forces to prepare for flood season and review Emergency Operation Plan in November.

Thanks

A handwritten signature in black ink, appearing to read "M. L. Z. [unclear]", is positioned to the right of the "Thanks" text.

October 20, 2023

Mr. Michael Moncrief
MBK Engineers
455 University Ave. #100
Sacramento, California 95825

RE: GEOTECHNICAL ENGINEERING SERVICES FOR SEEPAGE AREAS ON CACHE SLOUGH LEVEE, RECLAMATION DISTRICT 2084, SOLANO COUNTY, CALIFORNIA

Dear Mr. Moncrief:

INTRODUCTION

This letter presents a summary of our field observations, laboratory test results, and conclusions and recommendations regarding three seepage areas along the Reclamation District 2084 (District) levee adjacent to Cache Slough. The seepage areas are located at approximate District Stations 82+50 to 83+00, 214+50 to 220+00, and 276+00 to 278+00. The approximate locations of the seepage areas are presented on Figures 1 through 4.

Our scope of services was outlined in our proposal dated August 21, 2023. Our scope of services consisted of conducting a geotechnical investigation that included logging test pits, performing laboratory testing, and developing conclusions and recommendations for a remediation scheme that can be implemented this fall. The results of our geotechnical investigation are presented in this letter.

DATA REVIEW, FIELD EXPLORATION, AND LABORATORY TESTING

We reviewed logs of previous subsurface explorations within the proposed project footprint and surrounding vicinity. We also reviewed a published geologic map for the site (Atwater 1982) and a soil conservation map (NRCS 2018) for the site. Selected references are presented at the end of this letter.

We explored subsurface conditions on August 22 and 23, 2023 by excavating 12 test pits to approximate depths of 3 to 12 feet below existing grade. The test pits were excavated near the landside levee toe and on the landside slope. An excavator provided by the District was used to excavate the test pits. Our engineer logged the test pits and collected samples from the test pits for further visual classification and for selection of materials for laboratory testing. The test pits were loosely backfilled with the excavated material and tamped with the excavator bucket. The locations of the test pits are presented on Figures 2 through 4 and

summarized in Table 1. Descriptions of the materials encountered in the test pits are presented in Table 2.

Laboratory tests were performed on selected samples from the test pits. The laboratory testing program consisted of moisture content measurements, Atterberg Limits, and sieve analysis. The laboratory test results are presented in Table 2.

SITE CONDITIONS

Surface Conditions

The surface conditions vary at the three sites. For the remainder of this letter, Station 82+50 to 83+00 will be referred to as the south site, Station 214+50 to 220+00 will be referred to as the middle site, and Station 276+00 to 278+00 will be referred to as the north site. Elevations throughout this letter will be referenced to the North American Vertical Datum of 1988 (NAVD88).

At the north site, the levee crest elevation is approximately 15 feet, and the crest width is approximately 25 feet. An approximately 24-foot-wide ramp is located on the landside slope at approximately Elevation 3 feet. The inclination of the landside slope is approximately 3H:1V above the ramp and varies from approximately 2H:1V and 4H:1V below the ramp. The landside levee toe is at approximately Elevation -1.5 feet. Sandbag rings are present at the toe in locations where sand boils had previously been observed. We observed tules, puddles, and wet soil near the landside levee toe at the location indicated on Figure 2.

At the middle site, the levee crest elevation is approximately 14 feet and the crest width is approximately 24 feet. The inclination of the landside slope varies from approximately 3.5H:1V to 4.5H:1V. The elevation of the landside toe is approximately 0 feet. Sandbag rings are present at the levee toe in locations where sand boils had previously been observed. We observed tules, puddles, and wet soil near the levee toe and lower portions of the landside slope between approximately Stations 216+00 and 218+00, as indicated on Figure 3. The ground surface was moist between approximately Stations 214+50 and 220+00.

At the north site, the levee crest elevation is approximately 15 feet, and the crest width is approximately 24 feet. The inclination of the landside slope varies from approximately 3H:1V to 4.5H:1V. An unpaved road is cut into the landside levee toe at approximately Elevation -2.5 feet. The road is approximately 15 feet wide, and the vertical offset of the cut varies from 1 to 2 feet. We observed standing water on the ground surface on the toe road

and small tules growing at the levee toe at the location shown on Figure 4. Two ditches are located landward of the toe road which run parallel to the levee. The ditches contained water and were vegetated with tules and grasses during our visit.

Tide levels for Cache Slough near the project site are presented in Exhibit 1 below.

Exhibit 1: Tide Levels Near Project Site, from MBK (2022)

Tide Level	Elevation, feet (NAVD88)
Mean Higher High Water (MHHW)	6.5
Mean High Water (MHW)	5.9
Mean Tide Level (MTL)	4.4
Mean Low Water (MLW)	2.6
Mean Lower Low Water (MLLW)	2.1

Subsurface Conditions

Subsurface conditions at the south site generally consist of a low to moderate plasticity, lean clay embankment constructed on high-plasticity organic clay marsh soils. Groundwater was encountered at depths 7 to 10 feet below the ground surface.

Subsurface conditions at the middle and north sites generally consist of low-plasticity silt, sandy silt, and silty sand embankment fill over high-plasticity, organic clay marsh soils. The elevation of the interface between the fill and native soils was within approximately 2 feet of the elevation of the landside levee toe. The low-plasticity soils contained a fines content (percentage passing the No. 200 sieve) ranging from 50 to 94%. The organic clay marsh soils had liquid limits ranging from 51 to 68 and plasticity indices ranging from 25 to 37. Groundwater was encountered in all the pits except for Test Pit 11 and was generally observed near the interface of the embankment fill and native organic clay soils. The surface of the organic clay soils at the interface appeared cracked, loose, and/or disturbed, indicating that minimal ground preparation was performed before placing the embankment fill.

The above descriptions of soil and groundwater conditions summarize observations at the time of the investigations. The test pits were backfilled shortly after excavation and stabilized water levels were not obtained. Conditions are expected to vary across the site,

with time, and depend on several factors including changes in moisture content resulting from seasonal precipitation, irrigation practices, and tides.

CONCLUSIONS AND RECOMMENDATIONS

Water has been observed flowing from the landside levee slope near the levee toe on multiple occasions and we understand that the flow rate is greater during high water events. The water appears to be flowing through the levee near the interface of the embankment fill and the native soils at the middle and north sites. The path of water at the south site was not readily apparent from the test pits. The flow of water exiting the landside face and toe is undesirable. We conclude that remedial measures should be taken to reduce the seepage flowing through and beneath the levee. There are several options for interrupting or collecting the seepage flow with varying cost and reliability. We considered three options noted below.

1. Construct a berm at the levee toe with the intent to contain seepage below the top of the berm.
2. Internal drainage to collect seepage, and
3. A cutoff through the levee to reduce rates of seepage.

Typical cutoff methods, including soil-bentonite walls and sheet pile walls installed through the levee crest, cannot be completed this year. Option 3 can be implemented this year. It is a relatively low-cost maintenance activity. We conclude that a practical approach would be to cut off the seepage by excavating a trench through the levee near the midslope that extends below the bottom of the embankment fill and backfilling the trench with compacted soil.

The value of the trench is to disrupt through-going horizontal layers of permeable or erodible soil and attempt to block the path of seepage. The trench is not a low-permeability core. The seepage could still flow over, below, or through the compacted trench. The levee will need to be monitored after completion of the trench and further remediation may be necessary, depending on the effectiveness of the trenching.

Our recommended trench configuration is presented on Figure 5. The trench should be 3 feet wide. The top of the trench should be at or near Elevation 5 feet. The trench should extend at least 50 feet beyond observed seepage areas. The trench should extend as deep as practical without causing sloughing of the trench walls or extensive cracking of the levee crest adjacent to the trench. The depth of excavation should not exceed 8 feet unless authorized by us during construction. Excavated spoils may be used to backfill the exploratory trench. Backfilled material should be placed and compacted with a sheepsfoot

compactor, such as a wheel adapter for the excavator. The material should be placed in lifts less than 12 inches thick, moisture conditioned to at least optimum moisture content, and compacted to at least 90 percent relative compaction. At no time should there be more than 25 feet of trench open.

It was a pleasure working with you on this project and we look forward to working with you during construction. If you have any questions, please call.

Sincerely,

SHANNON & WILSON



Greg Olsen, PE, GE
Senior Engineer



R. Kevin Tillis, PE, GE
Vice President

GRO:RKT/kxb

Enc. Reference List

Table 1 – Locations of Test Pits
Table 2 – Logs of Test Pits
Figure 1 – Vicinity Map
Figure 2 – Site Plan: South Site
Figure 3 – Site Plan: Middle Site
Figure 4 – Site Plan: North Site
Figure 5 – Typical Detail – Landside Trench
Important Information About Your Geotechnical Report

REFERENCE LIST

Atwater, Brian F. 1982. United States Department of the Interior Geological Survey, Geologic Maps of the Sacramento-San Joaquin Delta, California.

MBK Engineers. 2022. Technical Memorandum, Cache Slough Mitigation Bank (CSMB) – Conceptual Level Hydrologic & Hydraulic Analysis, dated October 14, 2022.

Natural Resources Conservation Service. 2018. Custom Soil Resource Report for Solano County, California, Version 12, dated September 14, 2018.

Shannon and Wilson. 2023. Geotechnical Data Report, Little Egbert Multi-Benefit Project, Solano County, California, dated June 19, 2023.

Table 1: Locations of Test Pits

PIT #	DATE	LATITUDE	LONGITUDE	ELEVATION	LOCATION NOTES
1	8/22/23	38.189659	121.660924	5.5	On levee slope, adjacent to midslope ramp, on upslope side of ramp. Excavated perpendicular to slope.
2	8/22/23	38.189718	121.661015	-1.5	Landside levee toe, near sandbag ring at old boil. Excavated parallel to levee direction.
3	8/22/23	38.189735	121.66101	-1.5	Continuation of Test Pit 2 on landside levee toe, north side of Test Pit 2. Excavated parallel to levee direction.
4	8/22/23	38.222108	121.67493	0	Excavated at landside levee toe, perpendicular to levee slope direction.
5	8/22/23	38.222201	121.674947	3	Excavated adjacent to upslope face of Test Pit 4, approximately 3 feet above levee toe elevation, perpendicular to levee slope direction
6	8/22/23	38.222201	121.674909	6	Excavated adjacent to upslope face of Test Pit 5, approximately 6 feet above levee toe elevation, perpendicular to levee slope direction
7	8/22/23	38.222547	121.675065	5	Approximately 50-foot-long trench with upslope end of trench approximately 10 feet above levee toe and downslope end of trench approximately 10 feet past levee toe. Excavated perpendicular to levee slope direction. Depths and elevations referenced to approximate midpoint of trench.
8	8/23/23	38.235239	121.684061	-2.5	Excavated on toe road, between levee toe and landside ditch. Excavated perpendicular to slope.
9	8/23/23	38.235231	121.684071	1.5	Excavated adjacent to upslope face of Test Pit 8, approximately 4 feet above levee toe elevation, perpendicular to levee slope direction.
10	8/23/23	38.235231	121.684071	3.5	Excavated adjacent to upslope face of Test Pit 9, approximately 6 feet above levee toe elevation, perpendicular to levee slope direction.
11	8/23/23	38.235315	121.684265	-2.5	Excavated on toe road, between levee toe and landside ditch. Excavated perpendicular to slope.
12	8/23/23	38.235332	121.684211	2.5	Excavated adjacent to upslope face of Test Pit 11, approximately 5 feet above levee toe elevation, perpendicular to levee slope direction.

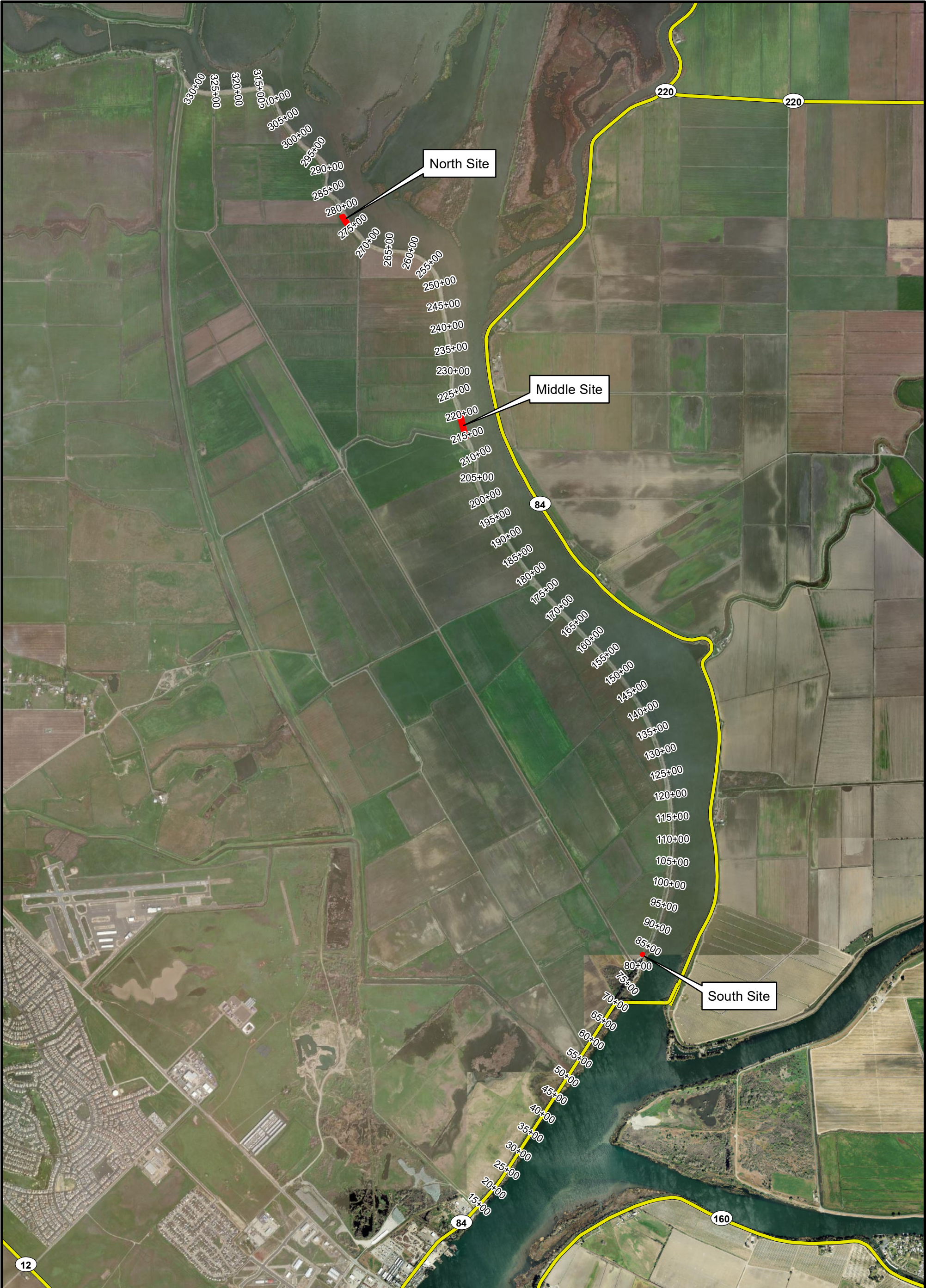
Table 2: Logs of Test Pits

PIT #	DEPTH (FT)	MATERIALS DESCRIPTION	LAB RESULTS
1	0 – 5.8	LEAN CLAY (CL), olive, dry to moist, stiff, low to medium plasticity, medium toughness, trace fine sand. (Fill).	Moisture Content = 18.2% (at depth 2 -2.5 ft).
	5.8 –10	ORGANIC CLAY (OH), brown, moist, stiff, medium to high plasticity.	Moisture Content = 37.5% (at depth = 6.0 ft).
	10 - 12	ORGANIC CLAY (OH), gray- brown, moist to wet, medium stiff to stiff, high plasticity, high toughness (native).	Moisture Content = 53.9% (at depth = 11.5 ft).
	10	Groundwater encountered at 10 ft	
2	0 - 8	ORGANIC CLAY (OH), gray-brown, moist at surface, becomes wet at 6 ft, high plasticity, medium stiff.	
	7	Groundwater encountered at 7 ft, Water Seeping into trench at multiple locations between 7 and 8 ft	
3	0 - 9	ORGANIC CLAY (OH), gray-brown, moist, becomes wet at 6 ft, high plasticity, medium stiff, organic fibers abundant in clay matrix.	At depth = 2.0 ft: Moisture Content = 35.1% Liquid Limit = 53 Plastic Limit = 27 Plasticity Index = 26
	7.5	Groundwater encountered at 7.5 ft, water seeping in at 7.5 to 8 ft.	
4	0 - 8	ORGANIC CLAY (OH), brown (0-1 ft), becomes olive gray at 1 ft, medium stiff to stiff, moist, becomes wet at 4 ft, high plasticity, high toughness. Trace fine grained sand in clay matrix at 4 ft to 6 ft No discrete sand layers observed. Grades silty at 6 ft	
	3	Groundwater seepage observed at 3 ft on waterside face of trench	

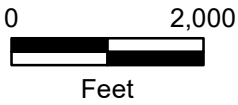
PIT #	DEPTH (FT)	MATERIALS DESCRIPTION	LAB RESULTS
5	0 - 2	SANDY SILT (ML), yellow brown, moist to wet, medium stiff, low plasticity (fill)	At depth = 0–2 ft: Moisture Content = 42% 100% < #4 Sieve 98% < #40 Sieve 64% < #200 Sieve
	2 - 8	ORGANIC CLAY (OH), olive gray, medium stiff to stiff, high plasticity	At depth = 3 - 3.5 ft: Moisture Content = 49.6% Liquid Limit = 68 Plastic Limit = 31 Plasticity Index = 37 100% < #4 Sieve 100% < #40 Sieve 98% < #200 Sieve
	1	Water flowing into test pit approximately 1 foot above interface between Sandy Silt and Organic Clay	
6	0 - 3	SILT (ML), brown, moist, stiff, low plasticity, low toughness, trace sand (fill)	At depth = 1–1.5 ft: Moisture Content = 34.5% 100% < #4 Sieve 99% < #40 Sieve 95% < #200 Sieve
	3 - 3.5	LEAN CLAY (CL), yellow brown, moist, stiff, (fill)	
	3.5 - 4	SILT with SAND (ML), yellow brown, wet, medium stiff, low plasticity, rapid dilatancy, (fill)	At depth = 3.5–4 ft: Moisture Content = 44.7% 100% < #4 Sieve 99% < #40 Sieve 83% < #200 Sieve
		Thin layer of black peat at 4 ft, approximately 1 inch thick	
	4 – 8.5	SILT with SAND (ML), yellow brown, wet, stiff, low plasticity, rapid dilatancy	At depth = 5.0 ft: Moisture Content = 43.3% Liquid Limit = 47 Plastic Limit = 30 Plasticity Index = 17 100% < #4 Sieve 98% < #40 Sieve 82% < #200 Sieve
	8.5 - 9	ORGANIC CLAY (OH), gray-brown, wet, medium stiff to stiff, high plasticity	
	4	Groundwater observed	

PIT #	DEPTH (FT)	MATERIALS DESCRIPTION	LAB RESULTS
7	0 - 2	SILT (ML), yellow brown, moist, stiff, low plasticity, low toughness (fill)	
	2 - 6	SANDY SILT (ML), yellow brown, wet, non-plastic to low plasticity, fine grained sand, medium stiff (trench was caving in this layer)	
		(Possible fill or native bar deposit)	
	6-8	ORGANIC CLAY (OH), gray-brown, wet, medium stiff to stiff, high plasticity	
	6	Groundwater encountered	
8	0 - 1	SILT (ML), yellow brown, moist to wet, stiff, trace fine sand, low plasticity, (fill)	
	1 - 3	ORGANIC CLAY (OH), gray-brown, moist to wet, high plasticity, medium stiff to stiff	At depth 2 – 3 ft: Moisture Content = 54.8%
	2	Groundwater encountered at 2 ft	
9	0 - 4	SILT (ML), yellow brown, moist, becomes wet at layer bottom, stiff, trace fine sand, low plasticity, observed glass bottle at 2 ft (fill)	At depth = 2 ft: Moisture Content = 42.7% Liquid Limit = 48 Plastic Limit = 30 Plasticity Index = 18 100% < #4 Sieve 99% < #40 Sieve 96% < #200 Sieve
	4 - 5	ORGANIC CLAY (OH), gray-brown, wet, medium stiff, high plasticity	
	4	Groundwater encountered at 3.5 ft	
10	0 - 6	SILT (ML), yellow brown, moist, stiff becomes medium stiff at bottom of layer, low plasticity, trace fine sand (fill)	
	6 – 7	ORGANIC CLAY (OH), gray-brown, wet, medium stiff, high plasticity	At depth = 7.0 ft: Moisture Content = 46.5% Liquid Limit = 51 Plastic Limit = 26 Plasticity Index = 25
	6	Groundwater encountered at 6 ft	
11	0 - 1	SANDY SILT (ML), moist, becomes wet at bottom of layer, medium stiff, fine-grained sand (fill)	At depth = 0 – 1 ft: Moisture Content = 29.9% 100% < #4 Sieve 94% < #40 Sieve 59% < #200 Sieve
	1 - 3	ORGANIC CLAY (OH), gray-brown, wet, medium stiff to stiff, high plasticity	

PIT #	DEPTH (FT)	MATERIALS DESCRIPTION	LAB RESULTS
12	0 - 6	SILTY SAND (SM), yellow brown, moist, becomes wet at bottom of layer, loose, fine-grained sand, (fill)	At depth = 2 ft: Moisture Content = 36.4% 100% < #4 Sieve 98% < #40 Sieve 50% < #200 Sieve
	6 - 8	ORGANIC CLAY (OH), gray-brown, wet, soft to medium stiff, high plasticity, Surface of layer is loose and disturbed. Ground likely not prepped before fill placed on top	
	6	Groundwater encountered at 6 ft	



 Approximate Location of Seepage Area



RD 2084 Seepage Areas
Solano County, California

VICINITY MAP

October 2023

112030

 SHANNON & WILSON

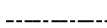
FIG. 1



Approximate Location of Test Pit



Approximate Limits of Standing Water at Ground Surface



District Stationing Centerline

N



0

30



Feet

RD 2084 Seepage Areas
Solano County, California

SITE PLAN - SOUTH SITE

October 2023

112030

 SHANNON & WILSON

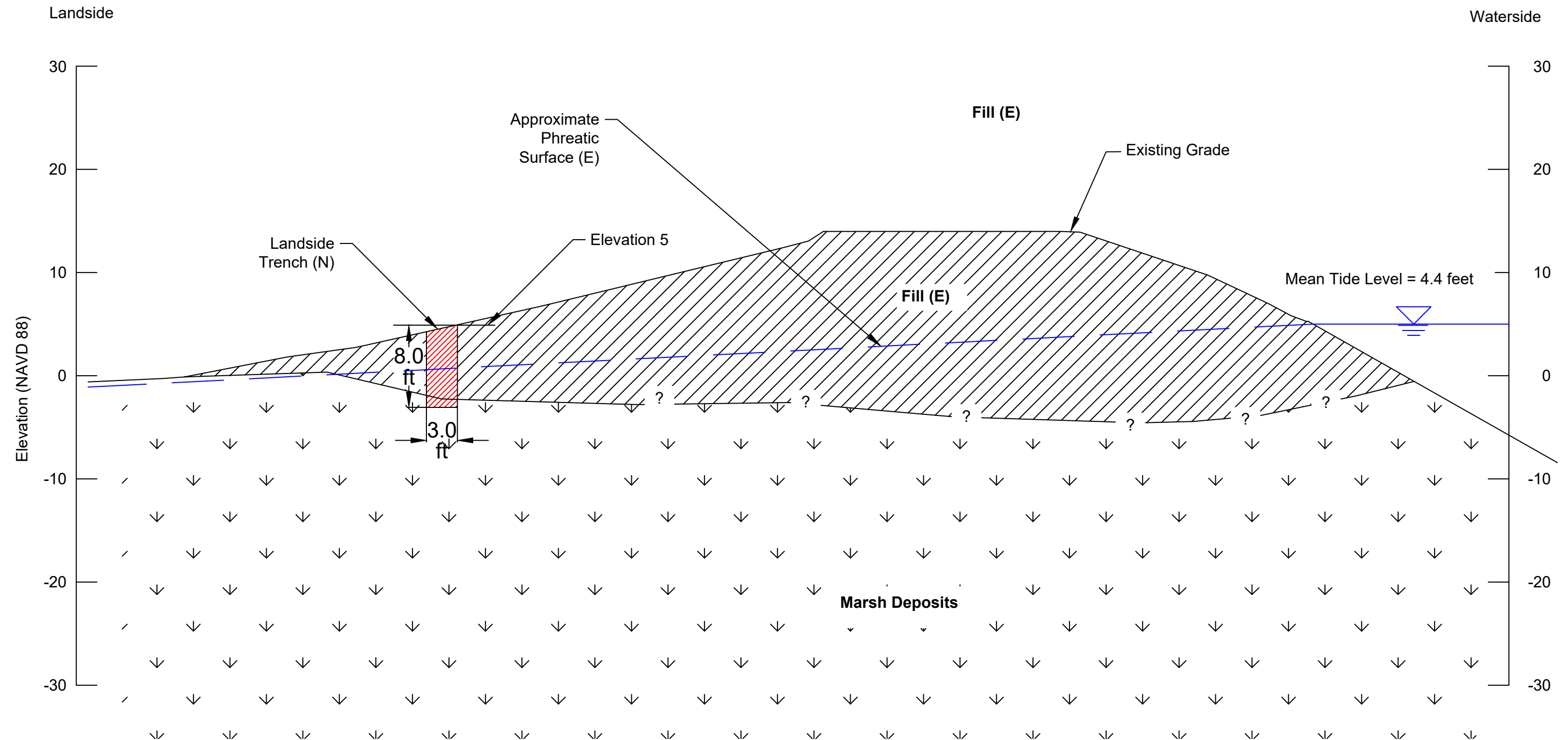
FIG. 2



<div><div><div><div><div></div><div></div></div><div></div></div><div>Approximate Location of Test Pit</div></div><div><div><div><div></div><div></div></div><div></div></div><div>Approximate Limits of Standing Water at Ground Surface</div></div><div><div><div><div></div><div></div></div><div></div></div><div>Approximate Limits of Moist Soil at Ground Surface</div></div><div><div><div><div></div><div></div></div><div></div></div><div>District Stationing Centerline</div></div></div>	<div><div>N</div><div></div></div> <div><div>0</div><div>60</div><div>Feet</div></div>	<div>RD 2084 Seepage Areas Solano County, California</div> <div><div>SITE PLAN - MIDDLE SITE</div><div>October 2023112030</div><div><div><div></div><div>SHANNON & WILSON</div></div><div>FIG. 3</div></div></div>
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<div><div><div></div><div>Approximate Location of Test Pit</div></div><div><div></div><div>Approximate Limits of Standing Water at Ground Surface</div></div><div><div></div><div>District Stationing Centerline</div></div></div>			<div>RD 2084 Seepage Areas Solano County, California</div>	
<div><div><div>N</div><div></div></div><div><div>0</div><div></div><div>40</div></div><div>Feet</div></div>			<div>SITE PLAN - NORTH SITE</div>	
			October 2023	112030
			<div><div></div>SHANNON & WILSON</div>	FIG. 4



NOTES

1. Shallow cutoff trench shall be excavated starting from the mid slope (approx EL + 5 ft), extending 8 feet deep.
2. Final configuration to be adjusted during construction.
3. Trench should extend through the fill into the native material.

NOT TO SCALE

RD 2084 Seepage Evaluation
Middle Site (Station 217+00)
Solano County, California

TYPICAL DETAIL LANDSIDE TRENCH

October 2023

112030

SHANNON & WILSON, INC.
GEOTECHNICAL AND ENVIRONMENTAL CONSULTANTS

FIG. 5

Important Information About Your Geotechnical/Environmental Proposal

More construction problems are caused by site subsurface conditions than any other factor. The following suggestions and observations are offered to help you manage your risks.

HAVE REALISTIC EXPECTATIONS.

If you have never before dealt with geotechnical or environmental issues, you should recognize that site exploration identifies actual subsurface conditions at those points where samples are taken, at the time they are taken. The data derived are extrapolated by the consultant, who then applies judgment to render an opinion about overall subsurface conditions; their reaction to construction activity; appropriate design of foundations, slopes, impoundments, and recovery wells; and other construction and/or remediation elements. Even under optimal circumstances, actual conditions may differ from those inferred to exist, because no consultant, no matter how qualified, and no subsurface program, no matter how comprehensive, can reveal what is hidden by earth, rock, and time.

DEVELOP THE SUBSURFACE EXPLORATION PLAN WITH CARE.

The nature of subsurface explorations—the types, quantities, and locations of procedures used—in large measure determines the effectiveness of the geotechnical/environmental report and the design based upon it. The more comprehensive a subsurface exploration and testing program, the more information it provides to the consultant, helping to reduce the risk of unanticipated conditions and the attendant risk of costly delays and disputes. Even the cost of subsurface construction may be lowered.

Developing a proper subsurface exploration plan is a basic element of geotechnical/environmental design that should be accomplished jointly by the consultant and the client (or designated professional representatives). This helps the parties involved recognize mutual concerns and makes the client aware of the technical options available. Clients who develop a subsurface exploration plan without the involvement and concurrence of a consultant may be required to assume responsibility and liability for the plan's adequacy.

READ GENERAL CONDITIONS CAREFULLY.

Most consultants include standard general contract conditions in their proposals. One of the general conditions most commonly employed is to limit the consulting firm's liability. Known as a "risk allocation" or "limitation of liability," this approach helps prevent problems at the beginning and establishes a fair and reasonable framework for handling them should they arise.

Various other elements of general conditions delineate your consultant's responsibilities. These are used to help eliminate confusion and misunderstandings, thereby helping all parties recognize who is responsible for different tasks. In all cases, read your consultant's general conditions carefully and ask any questions you may have.

HAVE YOUR CONSULTANT WORK WITH OTHER DESIGN PROFESSIONALS.

Costly problems can occur when other design professionals develop their plans based on misinterpretations of a consultant's report. To help avoid misinterpretations, retain your consultant to work with other project design professionals who are affected by the geotechnical/environmental report. This allows a consultant to explain report implications to design professionals affected by them, and to review their plans and specifications so that issues can be dealt with adequately. Although some other design professionals may be familiar with geotechnical/environmental concerns, none knows as much about them as a competent consultant.

OBTAIN CONSTRUCTION MONITORING SERVICES.

Most experienced clients also retain their consultant to serve during the construction phase of their projects. Involvement during the construction phase is particularly important because this permits the consultant to be on hand quickly to evaluate unanticipated conditions, conduct additional tests if required, and when necessary, recommend alternative solutions to problems. The consultant can also monitor the geotechnical/environmental work performed by contractors. It is essential to recognize that the construction recommendations included in a report are preliminary, because they must be based on the assumption that conditions revealed through selective exploratory sampling are indicative of actual conditions throughout a site.

Because actual subsurface conditions can be discerned only during earthwork and/or drilling, design consultants need to observe those conditions in order to provide their recommendations. Only the consultant who prepares the report is fully familiar with the background information needed to determine whether or not the report's recommendations are valid. The consultant submitting the report cannot assume responsibility or liability for the adequacy of preliminary recommendations if another party is retained to observe construction.

REALIZE THAT ENVIRONMENTAL ISSUES MAY NOT HAVE BEEN ADDRESSED.

If you have requested only a geotechnical engineering proposal, it will not include services needed to evaluate the likelihood of contamination by hazardous materials or other pollutants. Given the liabilities involved, it is prudent practice to always have a site reviewed from an environmental viewpoint. A consultant cannot be responsible for failing to detect contaminants when the services needed to perform that function are not being provided.

ONE OF THE OBLIGATIONS OF YOUR CONSULTANT IS TO PROTECT THE SAFETY, PROPERTY, AND WELFARE OF THE PUBLIC.

A geotechnical/environmental investigation will sometimes disclose the existence of conditions that may endanger the safety, health, property, or welfare of the public. Your consultant may be obligated under rules of professional conduct, or statutory or common law, to notify you and others of these conditions.

RELY ON YOUR CONSULTANT FOR ADDITIONAL ASSISTANCE.

Your consulting firm is familiar with several techniques and approaches that can be used to help reduce risk exposure for all parties to a construction project, from design through construction. Ask your consultant, not only about geotechnical and environmental issues, but others as well, to learn about approaches that may be of genuine benefit.

The preceding paragraphs are based on information provided by the GBA, Silver Spring, Maryland



**RECLAMATION
DISTRICT
BOARD**

Mark Young
President

Page Baldwin Jr.
Trustee

Marshall Cook
Trustee

Matt Gause
Trustee

Richard Harris
Trustee

October 27, 2023

Ms. Andrea Lobato, P.E., Manager
Delta Levees Program (Subventions)
Department of Water Resources
P.O. Box 942836
Sacramento, CA 94236-0001

**Subject: Reclamation District No. 2084 Delta Levees Maintenance
Subventions Program 2022-2023 Claim**

Dear Ms. Lobato:

In accordance with Part 1, Article 4, of the Delta Levees Maintenance Subventions Program Procedures and Criteria (August 26, 2016) and your letter dated February 8, 2023, Reclamation District No. 2084 hereby submits the attached final claim for reimbursement in the amount of \$86,410.78 for work accomplished under the Delta Levees Maintenance Subventions Program during fiscal year 2022-2023.

Sincerely,

Mark Young,
President Reclamation District 2084

cc: Central Valley Flood Protection Board (w/out enclosures)
Michael Moncrief, MBK Engineers (w/enclosure)
Madeline Baker, Larsen Wurzel & Associates (via email)
Tara Beltran, Reclamation District No. 2084

2022-2023 SUBVENTIONS PROGRAM

FINAL CLAIM

OCTOBER 2023

**RECLAMATION DISTRICT NO. 2084
LITTLE EGBERT TRACT**



Reclamation District No. 2084
Little Egbert Tract
Delta Levees Maintenance Subventions Program 2022-2023

FINAL CLAIM
October 2023

I. ROUTINE MAINTENANCE

A. Engineering Services - MBK Engineers (Exhibit A)

Services included coordination with District, contractor, funding and regulatory agencies, project planning, CEQA documentation, prepare Subventions application and claim, flood fight consultation, levee inspections, construction management, etc.

MBK ENGINEERS

July 2022	22-07-4290.1	585.00
August 2022	22-08-4290.1	3,296.75
September 2022	22-09-4290.1	1,358.50
October 2022	22-10-4290.1	3,586.75
November 2022	22-11-4290.1	2,286.25
December 2022	22-12-4290.1	2,155.00
January 2023	10259	1,920.05
February 2023	10423	2,722.17
March 2023	10677	3,639.19
April 2023	11010	3,764.75
May 2023	11293	10,511.53
June 2023	11501	8,899.80
		<hr/>
		\$ 44,725.74

B. Vegetation Control (Exhibit B)

Herbicide application on landside slope and crown roadway along perimeter levee.

Contracted Work

Miller Agriculture Invoice No. 800	8,650.00
Miller Agriculture Invoice No. 832	11,300.00
	<hr/>
	\$ 19,950.00

C. Levee Maintenance (Exhibit C)

The work covered by this invoice includes mobilization and the placement of approximately 240 tons of 18" minus quarry stone at 5 sites along the waterside

Contracted Work

Warren Gomes Excavating Invoice No. 3561	21,735.04
	<hr/>
	\$ 21,735.04

Final Claim 2022-2023 SUBVENTIONS **\$ 86,410.78**

**AB 360 PROGRAM
FUNDING CLAIM INFORMATION FORM**

This form must accompany all Subventions and Special Project Funding Claims for levee work under the AB 360 program. This includes progress claims, final claims, and emergency work (as soon after work completion as possible). This form is intended to summarize information necessary to comply with AB 360 mitigation/enhancement requirements. Use of this form will expedite the field inspection and payment process. It does not replace any other required AB 360 paperwork.

1. Claimant Information

- A. Reclamation District Name/Number: RD No. 2084, Little Egbert Tract
- B. Engineer: MBK Engineers
- C. Fiscal Year: 2022-2023

2. Work Description

Briefly describe the work actually performed. Identify locations by levee station and land/waterside as applicable. Include work dates. If there are various work locations, include the range of work stations. Provide pre-project and post-project photographs of any in-water work.

- A. Annual Routine Maintenance
 - i. Roads: None
 - ii. Erosion/Subsidence: None
 - iii. Drainage Control: None
 - iv. Toe Drain Cleaning: None
 - v. Vegetation Control: Herbicide application on landside slope and crown roadway along perimeter levee. (Exhibit B).
 - vi. Waterside Slope Protection: None
 - vii. Other: Levee Maintenance: Placement of approximately 240 tons of 18" minus quarry stone at 5 sites along the waterside slopes above mean-high water (Exhibit C).

- B. Levee Rehabilitation
 - i. HMP: None
 - ii. Bulletin 192-82: None
 - iii. Other: None

C. Emergency Work: None

D. Other: None

3. Mapping

See attached District Map with site locations identified.

4. Project Impacts

None.

SIGNATURES

Reclamation District No. 2084

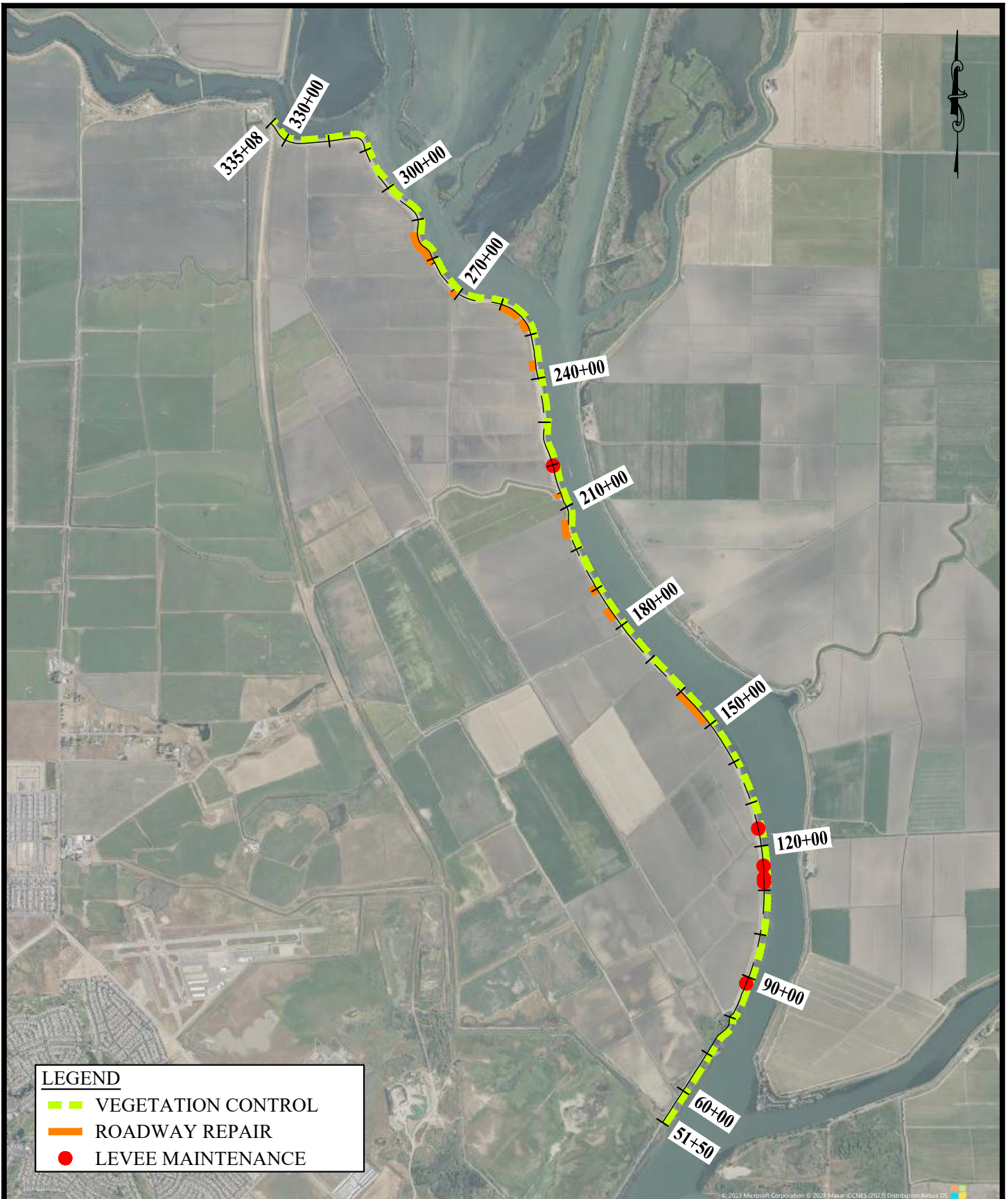
Department of Fish & Wildlife

District Engineer _____

Representative _____

Date Completed _____

Date Accepted _____



455 University Avenue, Suite 100
 Sacramento, California 95825
 Phone: (916) 456-4400 • Fax: (916) 456-0253

RECLAMATION DISTRICT NO. 2084 LITTLE EGBERT TRACT

2022-23 SUBVENTIONS

SCALE:	1" = 3000'
JOB NUMBER:	4575.6
DRAWN BY:	MN/AR
DATE:	10/24/2023
SHEET:	1 OF 1



**RECLAMATION
DISTRICT
BOARD**

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Trustee

Marshall Cook
Trustee

Matt Gause
Trustee

Richard Harris
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October 27, 2023

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cc: Central Valley Flood Protection Board (w/out enclosures)
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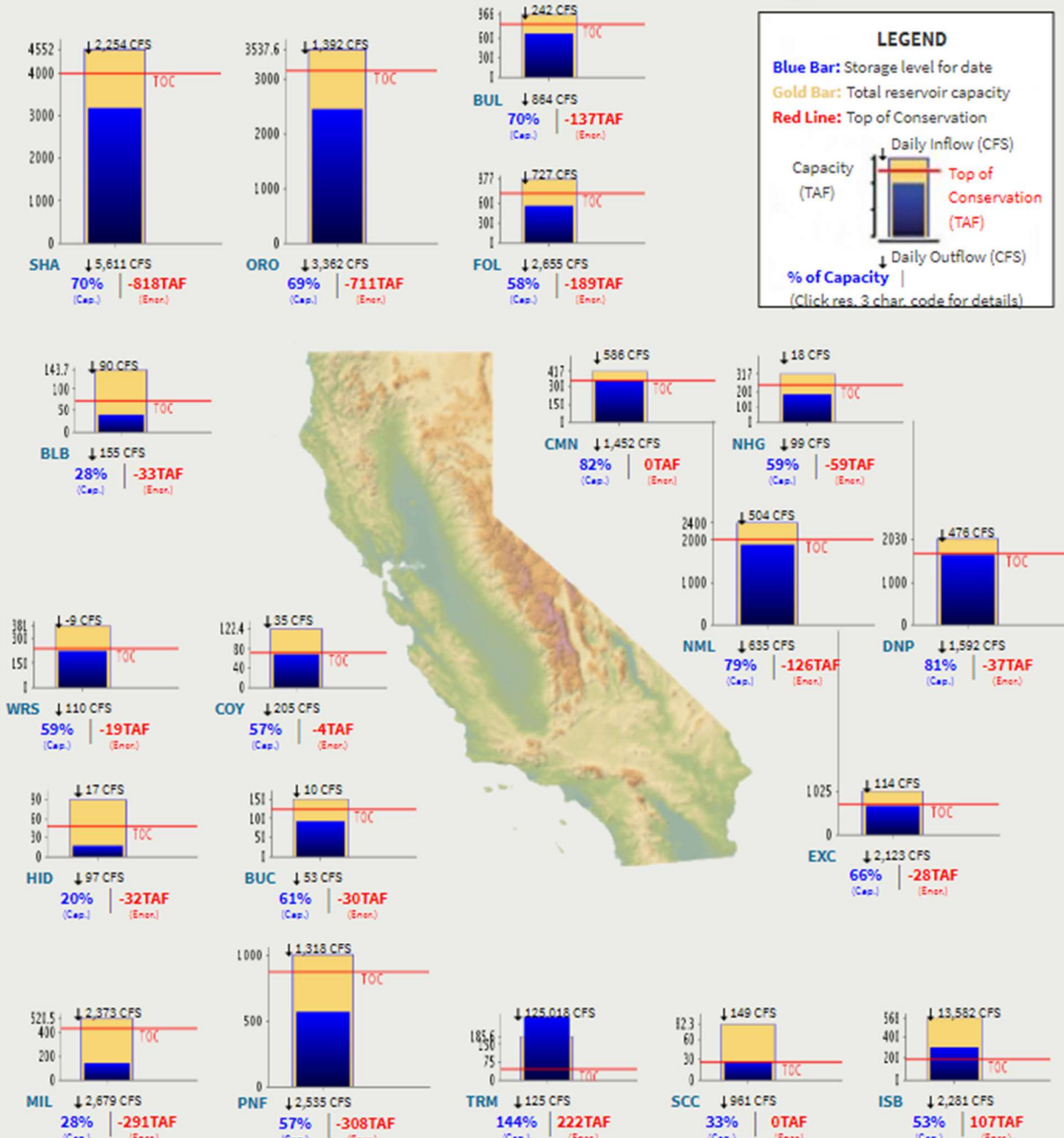
OCTOBER 27, 2023 – CURRENT CONDITIONS

TOP OF CONSERVATION CONDITIONS:

CENTRAL VALLEY AND RUSSIAN RIVER FLOOD CONTROL RESERVOIRS: 26-OCT-2023

Midnight: 26-Oct-2023

Change Date: 26-Oct-2023



[Click for printable version of current data.](#)

Report Generated: 27-Oct-2023 1:53 PM

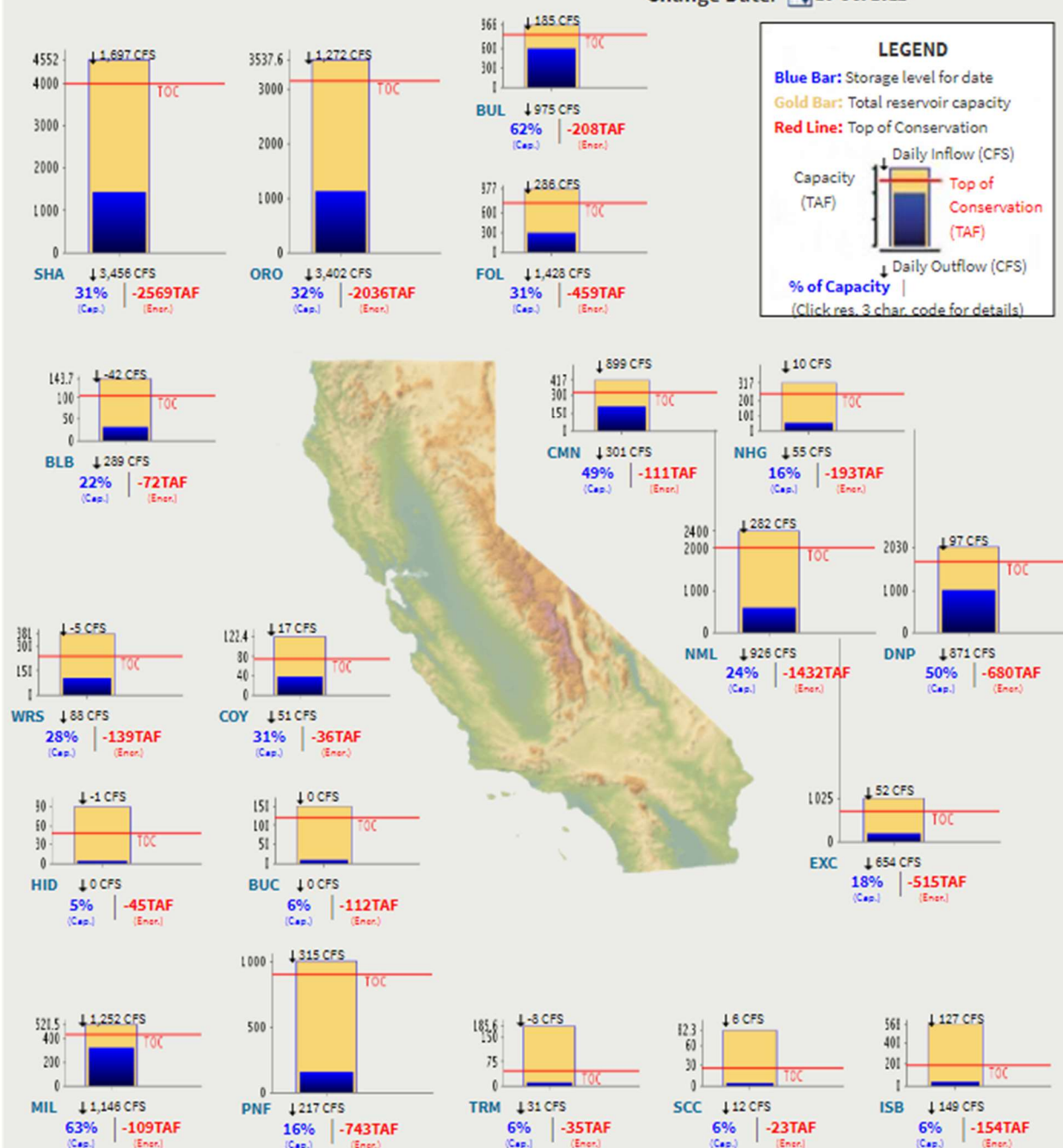
OCTOBER 27, 2022 – PRIOR YEAR CONDITIONS

TOP OF CONSERVATION CONDITIONS:

CENTRAL VALLEY AND RUSSIAN RIVER FLOOD CONTROL RESERVOIRS: 26-OCT-2022

Midnight: 26-Oct-2022

Change Date: 26-Oct-2022



Click for printable version of current data.

Report Generated: 27-Oct-2023 1:53 PM

ENCLOSURE 3

AGENDA ITEM 8.a



FINANCIAL MANAGER'S REPORT

Reclamation District 2084 Board of Directors

As of October 26, 2023

Paid Invoices	Invoices Pending	Total Invoiced
\$143,647.92	\$45,299.56	\$188,947.48

Current Budget	Less Invoice total	Budget Balance
\$626,079.00	\$188,947.48	\$437,131.52

Revenue Balance	Less Invoice total	Project Balance
\$198,943.00	\$188,947.48	\$9,995.52

Action Item

Informational Only.

Revenue	Vendor	Invoice #	Date	Amount
---------	--------	-----------	------	--------

Expenses	Vendor	Invoice #	Date	Amount
1	LWA	1912000-0623	09/22/2023	9,684.69
2	LWA	1912000-0723	09/22/2023	5,663.00
3	Richard Harris	014-22023	09/28/2023	6,000.00
4	LWA	1912000-0823	10/05/2023	3,468.00
5	PG&E	100523	10/05/2023	3,747.86
6	US Postal Service		10/19/2023	244.00
7	White Cap	Q55605130	10/19/2023	16,492.01

Fiscal Impact

District Invoices # 1-7 were reviewed and found to be consistent with the contract and within budget. The total invoiced amount approved between September and October is \$45,299.56

RECLAMATION DISTRICT 2084

Fiscal Year (FY) 2023-2024 (July 1st - June 30th)

REVENUES (Funds 100 and 200)

	Initial Budget	Revenues YTD	Current Receipts	Total Revenues
100100 Balance in Account (Carryover from 22/23) ^[4]	\$ 20,000.00	\$ 42,116.00	\$ -	\$ 42,116.00
100101A Assessment to Landowner	\$ 351,704.00	\$ 80,000.00	\$ -	\$ 80,000.00
100102 Five-Year Plan Funding	\$ -	\$ -	\$ -	\$ -
100103 Levee Subventions Program ^[1]	\$ 240,375.00	\$ 76,827.00	\$ -	\$ 76,827.00
100104 FEMA Emergency Funds	\$ 50,000.00	\$ -	\$ -	\$ -
TOTAL REVENUES (Funds 100 and 200)	\$ 372,079.00	\$ 198,943.00	\$ -	\$ 198,943.00

GO&A EXPENSES (Fund 100)

Personnel:

	Initial Budget	Prior Expenses	Current Expenses	Total Expenses
100201 President/Trustees/Officers	\$ -	\$ -	\$ -	\$ -
100202 Support Staff	\$ -	\$ -	\$ -	\$ -
100203 LEJPA Special Representative	\$ 18,000.00	\$ -	\$ 6,000.00	\$ 6,000.00
100204 Board Member Compensation ^[3]	\$ 9,600.00	\$ 1,496.96	\$ -	\$ 1,496.96
Subtotal Personnel	\$ 27,600.00	\$ 1,496.96	\$ 6,000.00	\$ 7,496.96

Administrative Contract Services:

100301 Administrative Support	\$ 70,800.00	\$ 13,189.36	\$ 18,815.69	\$ 32,005.05
100302 Legal Support	\$ 5,400.00	\$ -	\$ -	\$ -
100303a Engineering Support	\$ -	\$ -	\$ -	\$ -
100303b Engineering Support - Subventions ^[2]	\$ 50,000.00	\$ 24,862.68	\$ -	\$ 24,862.68
100304 Accounting	\$ 7,500.00	\$ -	\$ -	\$ -
100308 Five Year Plan Development	\$ 600.00	\$ -	\$ -	\$ -
Subtotal Administrative Contract Services	\$ 134,300.00	\$ 38,052.04	\$ 18,815.69	\$ 56,867.73

Services and Supplies (Excluding Consultant Expenses):

100502 PO Box Renewal	\$ 225.00	\$ -	\$ 244.00	\$ 244.00
100505 Website & Hosting	\$ 390.00	\$ -	\$ -	\$ -
100508c CCVFC - Dues	\$ 1,145.00	\$ -	\$ -	\$ -
100510 Liability Insurance	\$ 7,000.00	\$ 10,677.00	\$ -	\$ 10,677.00
100512 Bank Service Charges	\$ 5.00	\$ -	\$ -	\$ -
100513 CA SWRCB Annual Fee	\$ 400.00	\$ -	\$ -	\$ -
Subtotal Services / Supplies	\$ 9,165.00	\$ 10,677.00	\$ 244.00	\$ 10,921.00
TOTAL GO&A EXPENSES (Fund 100):	\$ 171,065.00	\$ 50,226.00	\$ 25,059.69	\$ 75,285.69

O&M EXPENSES (Fund 200):

200200 Levee Slope/Bench Mowing ^[2]	\$ 10,000.00	\$ -	\$ -	\$ -
200201 Rodent Control ^[2]	\$ 10,000.00	\$ -	\$ -	\$ -
200202 Levee Top & Access Road Maintenance ^[2]	\$ 75,000.00	\$ -	\$ -	\$ -
200203 Drainage Channel Clearing	\$ 5,000.00	\$ -	\$ -	\$ -
200204 Pump Station O&M	\$ 4,200.00	\$ -	\$ -	\$ -
200205 Electrical Power	\$ 41,250.00	\$ 8,446.95	\$ 3,747.86	\$ 12,194.81
200206 Misc. Supplies (pump oil, etc.)	\$ -	\$ -	\$ -	\$ -
200207 General Maintenance ^[2]	\$ -	\$ -	\$ -	\$ -
200208 Misc. O&M	\$ 2,000.00	\$ -	\$ -	\$ -
200209 Brush Removal/Herbicide ^[2]	\$ 50,000.00	\$ 9,200.00	\$ -	\$ 9,200.00
200210 Emergency Monitoring/Gaging and Response ^[2]	\$ 2,500.00	\$ -	\$ -	\$ -
200211 Environmental Permits	\$ -	\$ -	\$ -	\$ -
200212 Waterside Slope Maintenance ^[2]	\$ 125,000.00	\$ -	\$ -	\$ -
200213 WSM - Design & Permitting ^[2]	\$ 75,000.00	\$ -	\$ -	\$ -
200214 Remove or Modify Encroachments ^[2]	\$ 25,000.00	\$ -	\$ -	\$ -
TOTAL O&M EXPENSES (Fund 200):	\$ 424,950.00	\$ 17,646.95	\$ 3,747.86	\$ 21,394.81

EMERGENCY RESPONSE EXPENSES (Fund 400):

400204 Pump Station O&M	\$ 5,690.00	\$ 27,841.39	\$ -	\$ 27,841.39
400205 Electrical Power	\$ 21,911.00	\$ -	\$ -	\$ -
400206 Misc. Supplies (pump oil, etc.)	\$ -	\$ -	\$ -	\$ -
400207 General Maintenance	\$ -	\$ -	\$ -	\$ -
400210 Emergency Monitoring/Gaging and Response	\$ 2,463.00	\$ 47,933.58	\$ 16,492.01	\$ 64,425.59
TOTAL EMERGENCY RESPONSE EXPENSES (Funds 400):	\$ 30,064.00	\$ 75,774.97	\$ 16,492.01	\$ 92,266.98
TOTAL AGENCY EXPENSES (Funds 100 200 and 400):	\$ 626,079.00	\$ 143,647.92	\$ 45,299.56	\$ 188,947.48

Total Expenses YTD	Revenue less Expenses	Budget less Expenses
\$ 188,947.48	\$ 9,995.52	\$ 183,131.52

LEJPA

	Revenues Received	Budget Remaining
100101A Landowner Contributions (Revenue)	\$ 800,000.00	\$ 720,000.00
100311 Member Agency Assessment (LEJPA)	\$ 800,000.00	\$ -

[1] RD 2084 will be eligible for subvention funding for FY 23-24 expenses. State Reimbursements are assumed to occur in June of the following fiscal year.

[2] Expenses assumed eligible for State Subventions funding. Based on conversation with MBK.

[3] Includes expense reimbursements for RD2084 and LEJPA activities.

[4] Account includes carryover general funds and Subvention reimbursement from 22/23 costs.

[5] 400000 Series expenses anticipated to be 100% reimbursable by FEMA under DR-4683